



Lower Barataria Sediment Diversion

Restore Americas Estuary Conference
November 4, 2014





The Project

- Diversion - 10% Design Analyses
- Civil, Geotechnical, Structural and Modeling
- Environmental, Social, or Economic Impacts Not Considered



The Objective

- Identify the Most Cost Effective
- Most Environmentally Acceptable
- 50,000 cfs diversion when Mississippi River is at 1,000,000 cfs



Project Team

- ARCADIS
- Coastal Protection and Restoration Authority
- The Water Institute of the Gulf
- LSU

The Process

- 5 Sites Provided by TWI Through ongoing Hydrologic Modeling
- Key concept - Utilize Existing Data
- Minimal data collection



Civil Engineering

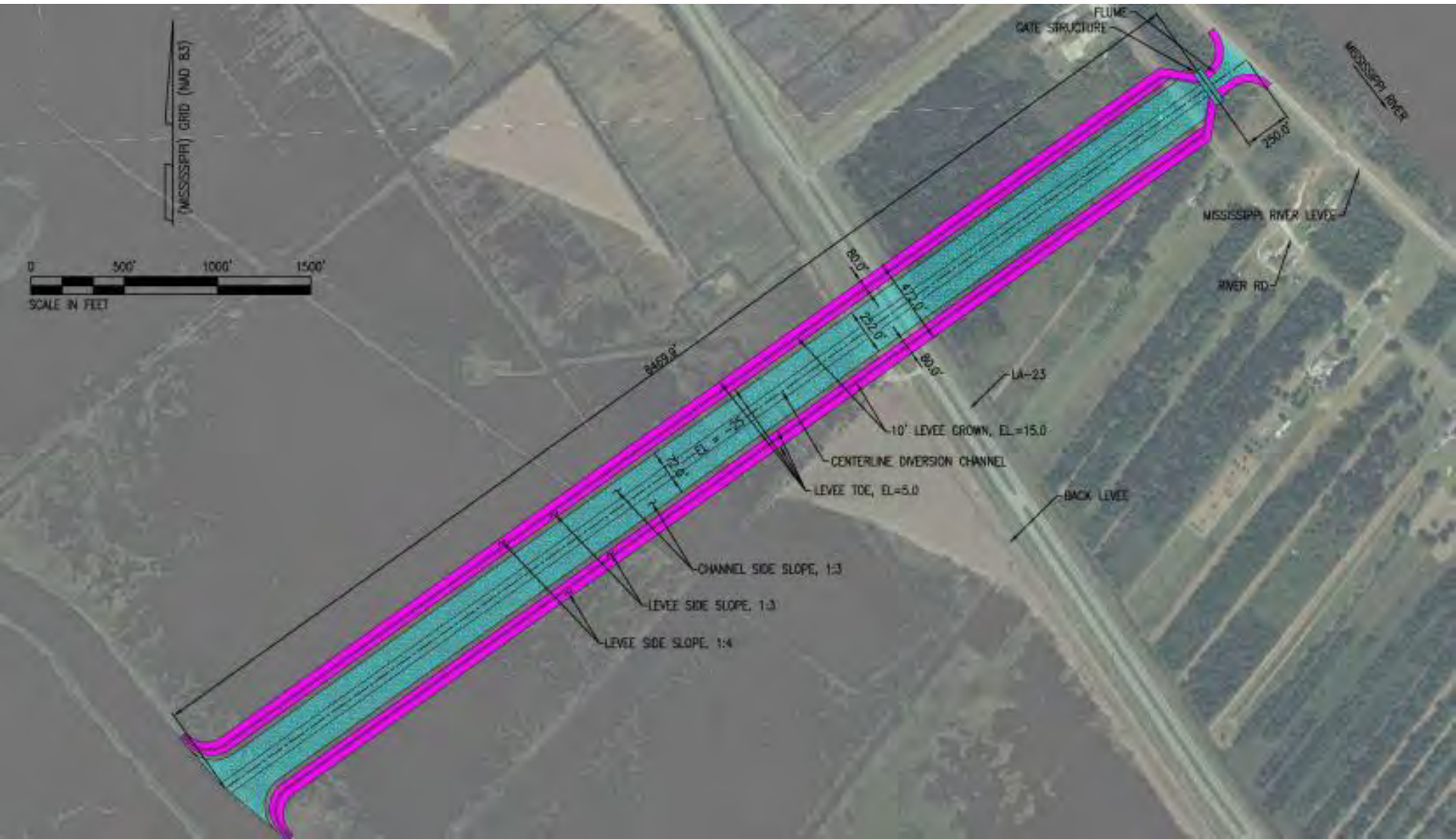
- Initial Site Layouts
- General Area of Preliminary Modeling Results
- Set Marshland Elevation/Channel Width and Length/Invert at Entrance



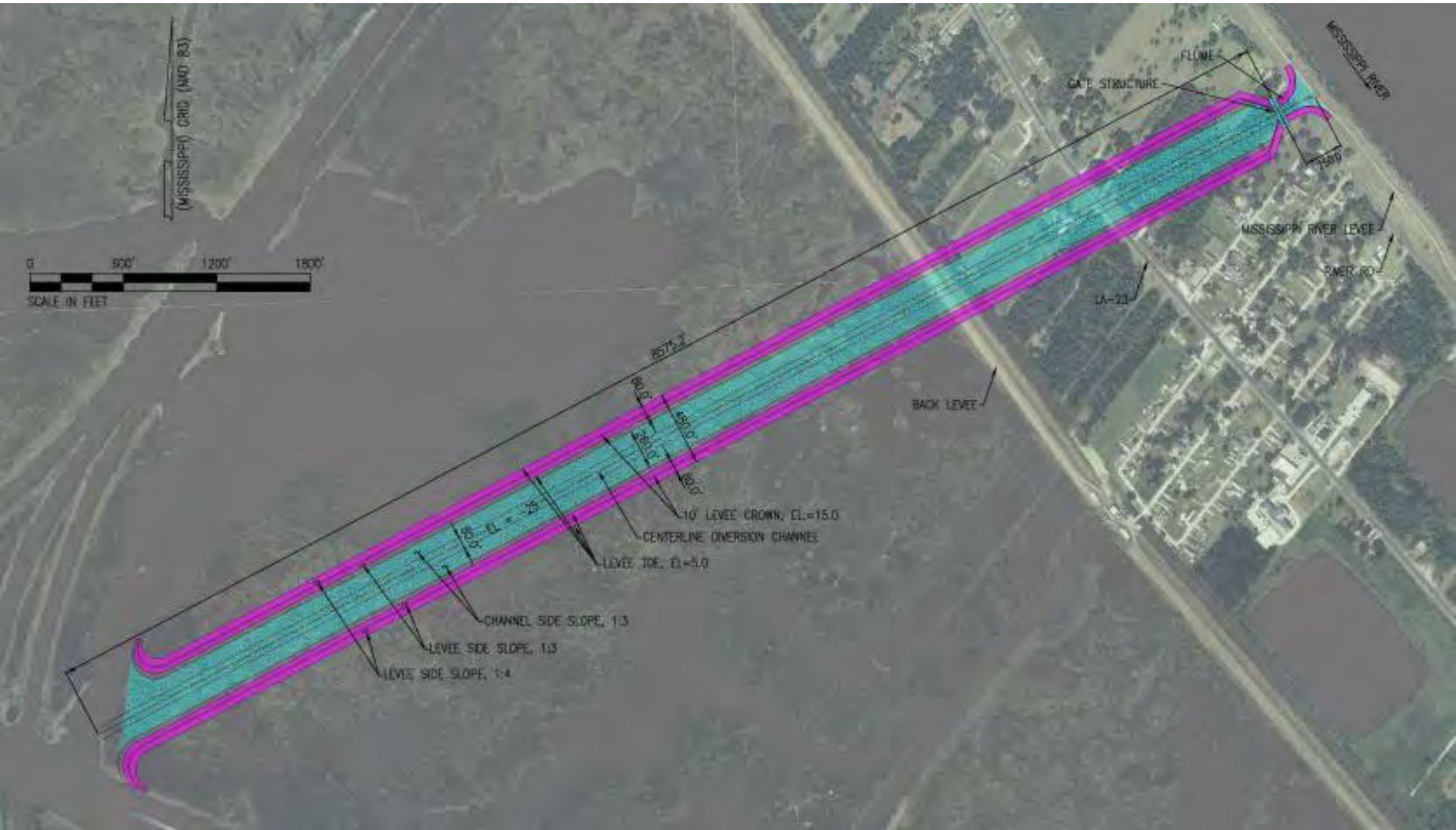


Source: The Water Institute of the Gulf.

Magnolia Concept Plan



Diamond Concept Plan



Port Sulphur Concept Plan



Empire Concept Plan



Buras Concept Plan



Location Summary

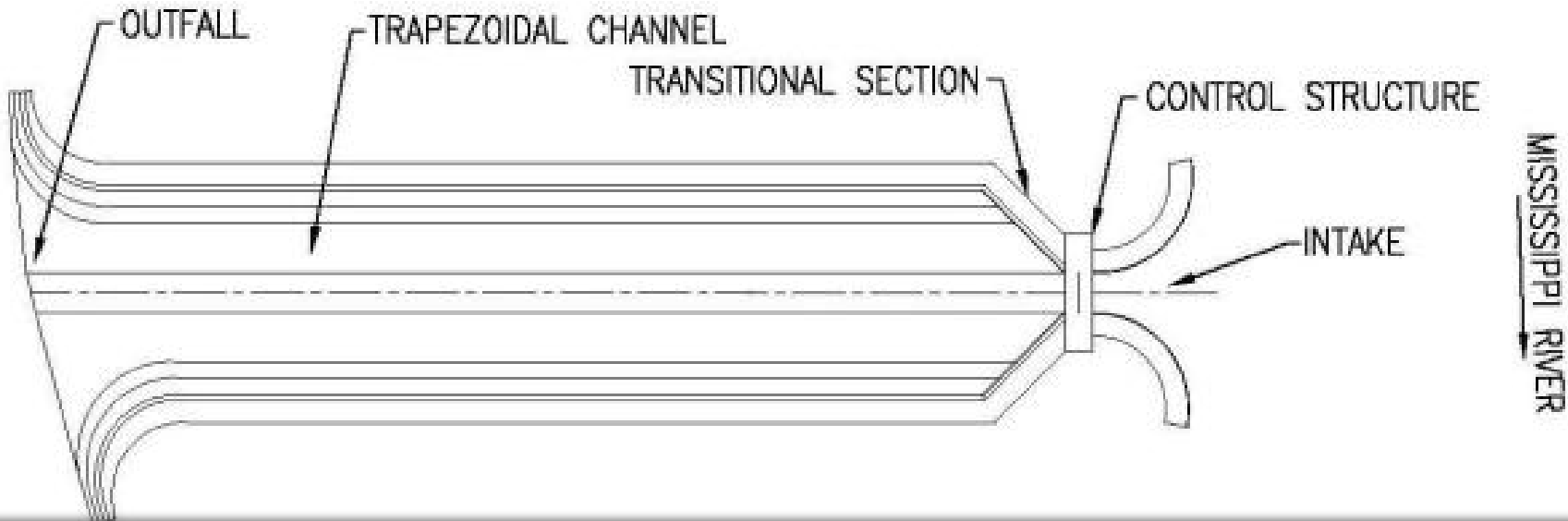
Site	Length (miles)	SWR (Cumulative)	Anchorage	Revetment	River Power	Road Crossings	Lane Crossings	Structures on Land	Special Features
Magnolia	1.46	1.11	Neg.	Pos.	1st	LA-23 (4 lanes) Diamond Rd (2 lanes)	6	none	
Diamond	1.88	0.99	Pos.	Neg.	2nd	LA-23 (2 lanes) River Rd (2 lanes)	4	3 mobile homes; 2 single family homes	
Port Sulphur	0.89	0.46	Pos.	Neg.	3rd	LA-23 (4 lanes)	4	mobile office trailers, metal building	existing large MR dock
Empire	0.83	0.71	Pos.	Neg.	4th	LA-23 (4 lanes) Frontages (4 lanes) Hwy 11 (2 lanes)	10	1 mobile home	existing LA-23 bridge
Buras	1.38	0.97	Neg.	Pos.	5th	LA-23 (4 lanes) Hwy 11 (2 lanes) River Rd (2 lanes)	8	2 mobile homes; 1 single family home; abandon gas station and metal bldg	



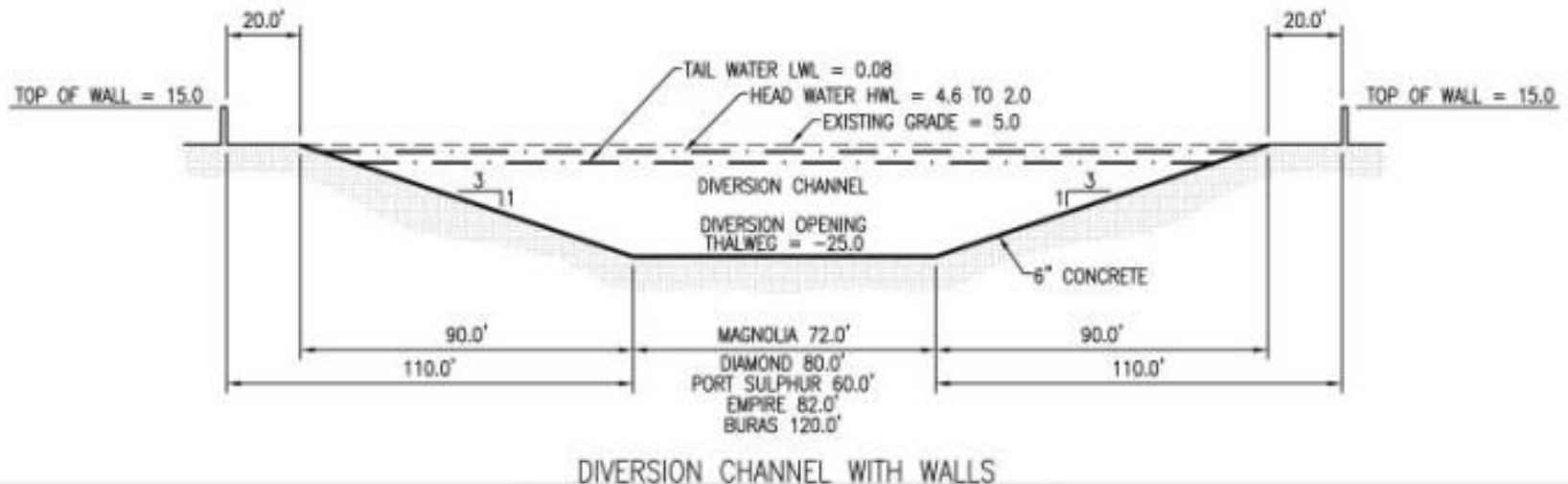
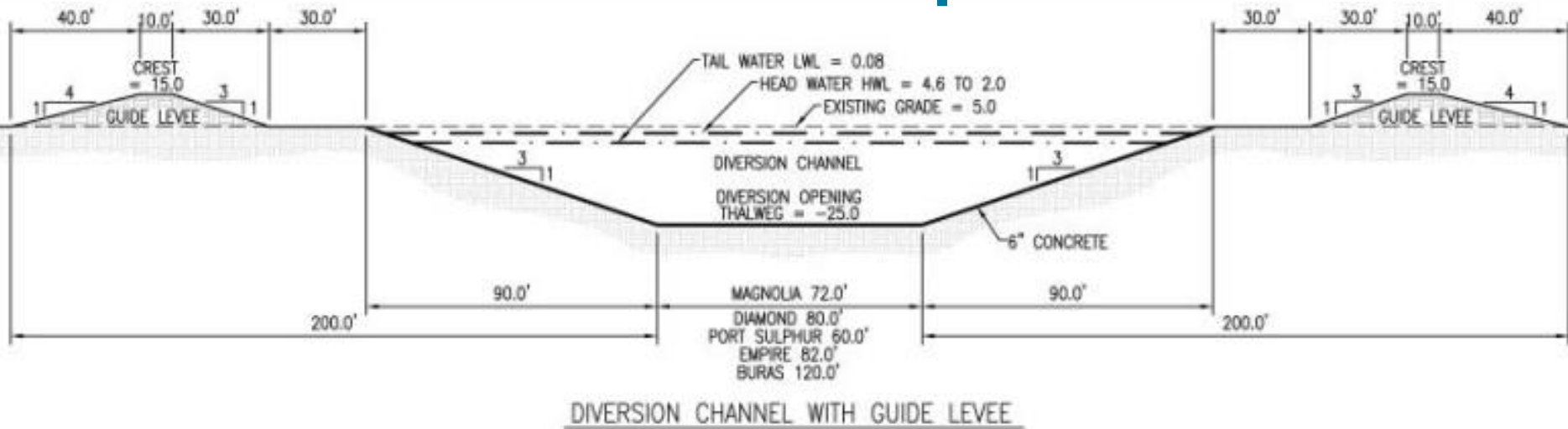
Design Criteria

Channel segments

1. Intake connection to Mississippi River
2. Control structure
3. Transition to trapezoidal channel
4. Trapezoidal channel
5. Outfall



Channel Cross Section Options



Channel Dimensions

Location	Invert	Width	Length	Upstream Water Surface Elevation	Downstream Water Surface Elevation	Average Water Surface Elevation
Magnolia	-25	72	7,700	4.6	0.08	2.4
Diamond	-25	80	9,878	4.2	0.08	2.1
Port Sulphur	-25	60	4,681	3.8	0.08	1.9
Empire	-25	82	4,400	2.8	0.08	1.4
Buras	-25	120	7,300	2.0	0.08	1.0

- Width is at bottom of channel
- Side slope = 3H : 1V
- Assume 2 feet head loss through control structure

Outstanding Design Considerations

- Control structure at back levee?
- Bridges required at roads other than LA-23?
- Guide walls OR guide levees?
- Disposal of excavation materials?
- Work needed beyond back levee tie-in?
- Tainter gates (multiple bays) with stop log system for maintenance

Design Assumptions

- Concrete channel lining
- Control structure with pile foundation
- Survey Data: Bathymetric data from ACOE revetment surveys along with LIDAR and USGS information will be utilized
- Hydraulic Data: Supplied by the WI.
- Guide Levee Elevation = 14.5

Geotechnical Engineering

- Reviewed existing reports and data
- Preliminarily analyzed levee and excavation slope stability for the highlighted sites:

Location	MRL	Back Levee
Magnolia	Nov. 9	Nov. 5
Diamond	Nov. 10	No Info (Nov. 6)
Port Sulphur	Nov. 10	No Info (Nov. 6)
Empire	Nov. 16	Nov. 7
Buras	Limited Info (Nov. 11)	Nov. 7

Geotechnical Engineering (cont.)

- Coordinated with the New Orleans USACE's PM
- Field located, staked and submitted permit requests to Plaquemines Parish, USACE and CPRA of proposed CPT locations
- CPT's will not be needed for Magnolia due to NOV 5 borings and Empire due to LADOTD Bridges at Hwy 11 and Hwy 23



Geotechnical Engineering (cont.)

- Slope Stability Analyses
- Pile Capacities
- Levee Sections/Wall Sections
- Cofferdams necessary for Construction

Geotechnical Design Assumptions

Material Unit Properties

Material	Unit Weight	Short Term, Cohesion	Short Term, ϕ	Long Term, ϕ
▪ Water	63 lb/ft ³	0 psf	0°	0°
▪ Rip rap	132 lb/ft ³	0 psf	40°	40°
▪ Silt	115 lb/ft ³	200 psf	15°	28°
▪ Silty Sand	122 lb/ft ³	0 psf	30°	30°
▪ Levee	110 lb/ft ³	400 psf	0°	23°

Pile Capacity Safety Factors

▪ Gate and Wall Structures	Preliminary and Final Analysis, FS = 2 (Assume Pile Load Tests)		
▪ Bridge Structure	Preliminary, FS = 2	Final, LRFD analysis	

Slope Stability Safety Factors

▪ Top of Levee	Preliminary and Final Analysis, Spencer's Method FS = 1.4 (Short-term strengths only)		
▪ Excavation	Preliminary and Final, Spencer FS = 1.4 (Short-term strengths only)	Additional Final, Spencer FS =1.4 & MOP= 1.3 (Long-term strengths)	
▪ Still Water Level (SWL)	Final Only, Spencer's Method FS = 1.5 (Short-term strengths), Spencer FS =1.4 & MOP= 1.3 (Long-term strengths)		

Geotechnical Design Assumptions (Cont.)

Design References

- *HSDRRSDG – Hurricane and Storm Damage Risk Reduction System Design Guidelines, USACE MVN
- EM 1110-2-1913, Design and Construction of Levees
- EM 1110-2-1902, Slope Stability
- EM 1110-2-2105, Design of Hydraulic Steel Structures
- EM 1110-2-2502, Retaining and Flood Walls
- EM 1110-2-2906, Design of Pile Foundations
- EM 1110-2-2504, Design of Sheet Pile Walls
- Engineering Circular (EC) 1110-2-6066, Design of I-walls
- EM 1110-2-1901, Seepage Analysis and Control for Dams
- Engineering Technical Letter (ETL) 1110-2-569, Design Guidance for Levee Underseepage
- Division Regulation (DIVR) 1110-1-400, Soil Mechanic Data
- AASHTO LRFD Bridge Design Specifications



Structural Engineering

- Obtained structural design information for similar projects: White Ditch, Myrtle Grove/Mid Barataria diversion
- Reviewed proposed gate types and proportions based on USACE guidelines

Structural Engineering

- Control Structure
- Back Levee Structure
- Bridge

Structural Design Assumptions

Material Unit Weights

▪ Water	63 lb/ft ³
▪ Reinforced Concrete	150 lb/ft ³
▪ Steel	490 lb/ft ³
▪ Rip rap	132 lb/ft ³
▪ Soil (dry, all type)	120 lb/ft ³

Material Specifications

▪ Structural Steel (Rolled Shapes)	ASTM A709, Grade 50 (50 ksi yield stress)
▪ Structural Steel (Channels, plates & others)	ASTM A36 (36 ksi yield stress)
▪ Steel Reinforcement	ASTM A615 Gr. 60 (60 ksi yield stress)
▪ Superstructure Concrete	4,000 psi at 28 days
▪ Pre-stressed Concrete	5,000 psi minimum
▪ Welding	As per AWS D1.1 and D1.5
▪ Anchor Bolts	ASTM A325 Type 1 for non-weathering steel and Type 3 for weathering steel

Structural Design Assumptions (Cont.)

Design Loads

▪ Structural Loading	As per USACE MVN HSDRRSDG* Section 5.7
▪ Wind	150 mph
▪ Seismic Loading	Not considered
▪ Impact Load	Corresponding to Zone #1B and Zone #3, as per HSDRRSDG, Section 5.2.1
▪ Bridge Load	LADOTD Louisiana Design Vehicle Live Load 2011 (LADV-11), Lane Load

Design References

- *HSDRRSDG – Hurricane and Storm Damage Risk Reduction System Design Guidelines, USACE MVN
- EM 1110-2-2104, Strength Design for Reinforced Concrete Hydraulic Structures
- EM 1110-2-2105, Design of Hydraulic Steel Structures
- EM 1110-2-2502, Retaining and Flood Walls
- EM 1110-2-2610, Lock and Dam Gate Operating and Control Systems
- American Concrete Institute, Building Code and Commentary, ACI 318-08
- American Institute of Steel construction, Manual of Steel Construction (9th Ed.)
- ASCE/SEI 7-10, Including Supplement No. 1, Minimum Design Loads for Buildings and Other Structures

Tainter Gates



Cost Estimating

- Determined preliminary project material and labor costs
- Divided project components and quantities
- Investigated real estate parcel boundaries and appraised values
- Contingency = 30%



Construction Cost Estimate - Schedule of Values

Item	Unit	Cost
Asphalt	Ton	\$55
Road Base	S.F.	\$15
Bridge	S.F.	\$130
30-inch Pipe Piles	L.F.	\$150
Reinforced Concrete	C.Y.	\$1,100
Steel	Ton	\$8,000
Excavation	C.Y.	\$60
Fill (guide levee, coffer dam, etc.)	C.Y.	\$35
Steel Sheet Pile	S.F.	\$35
Soilcrete	C.Y.	\$25

Real Estate Cost Estimate – Land Value Matrix

Category	Unit	Unit Value
Unimproved Acreage	Acre	\$8,000
Established Agricultural Acreage	Acre	\$15,000
Acreage with Structures/Homes	Acre	\$20,000
Previous Industrial Site	Acre	\$20,000
Structures Above Flood Level	S.F.	\$105
Structures Below Flood Level	S.F.	\$35
Single Family Homes Below Flood Level	S.F.	\$70
Outbuildings	S.F.	\$15

- Based on research of local land values
- No public GIS database available for Plaquemines Parish

Construction Cost Estimate – Site Summary

Site	Cost	Contingency	Total
Magnolia	\$679,370,285	\$203,811,086	\$883,181,371
Diamond	\$760,606,748	\$228,182,024	\$988,788,772
Port Sulphur	\$614,291,835	\$184,287,551	\$798,579,386
Empire	\$776,284,353	\$232,885,306	\$1,009,169,659
Buras	\$837,974,496	\$251,392,349	\$1,089,366,845



Overall Site Ranking

Category	Wtg Fac	Magnolia		Diamond		Port Sulphur		Empire		Buras	
		Rank	Score	Rank	Score	Rank	Score	Rank	Score	Rank	Score
Land Use Impacts	1.5	4	6	3	4.5	5	7.5	2	3	1	1.5
Channel Length	1	2	2	1	1	4	4	5	5	3	3
SWR	3	5	15	4	12	1	3	2	6	3	9
River Power	1	5	5	4	4	3	3	2	2	1	1
Bridges/ Structures	1	3	3	4	4	5	5	1	1	2	2
Soil Composition	2	5	10	4	8	2	4	1	2	3	6
Logistics	0.5	5	2.5	4	2	3	1.5	2	1	1	0.5
LA-23 Bypass Lane	1	4	4	3	3	5	5	1	1	2	2
Cost	2	4	8	3	6	5	10	2	4	1	2
Total Score			55.5		44.5		43		25		27

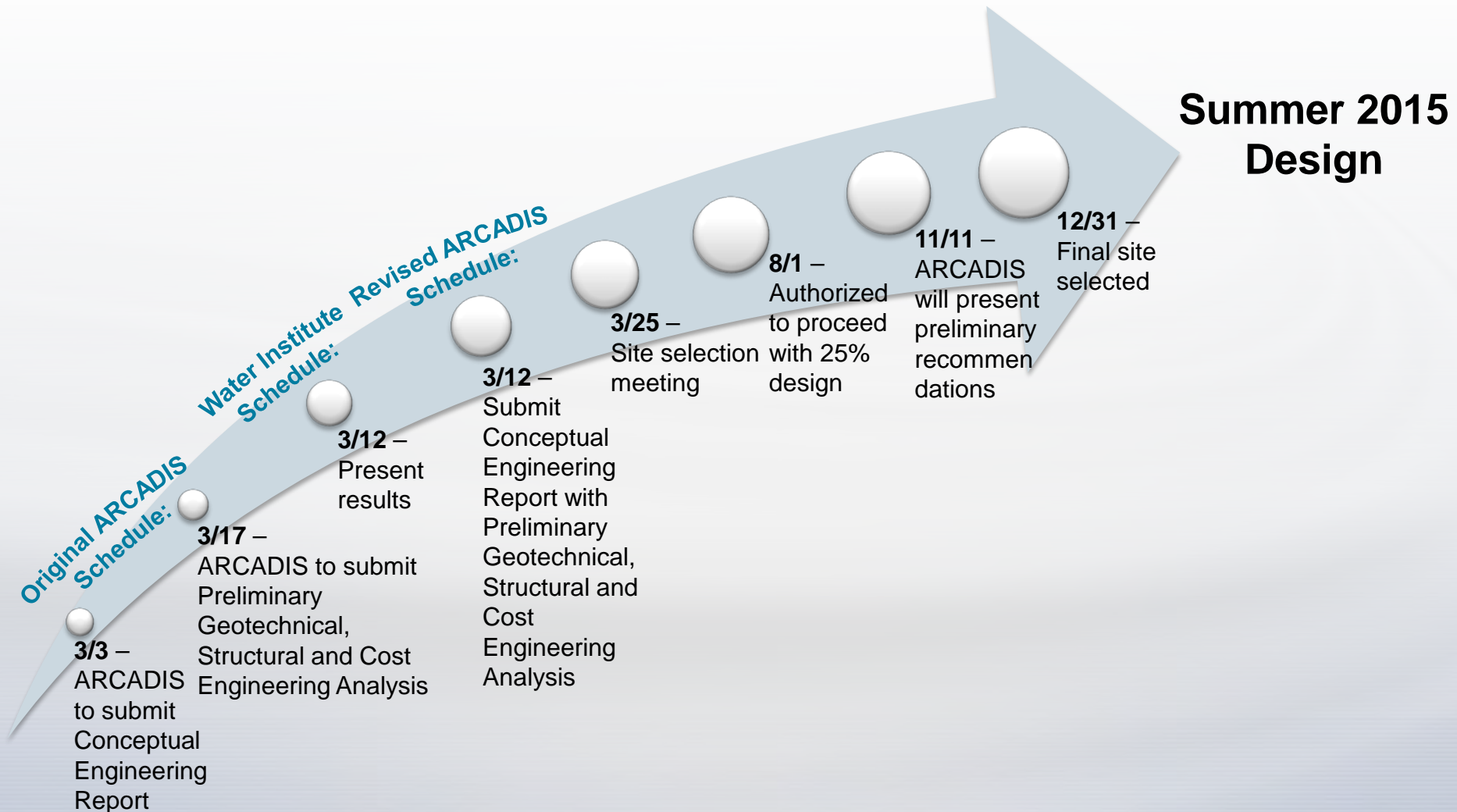
Reduced Sites to 2 Locations



Where Are We Now??

- 25% Design Effort – 2 Sites
- Flow 3D and HEC –RAS Modeling
- Refining Channel Components
- Refining Cost Estimating

Schedule Milestones



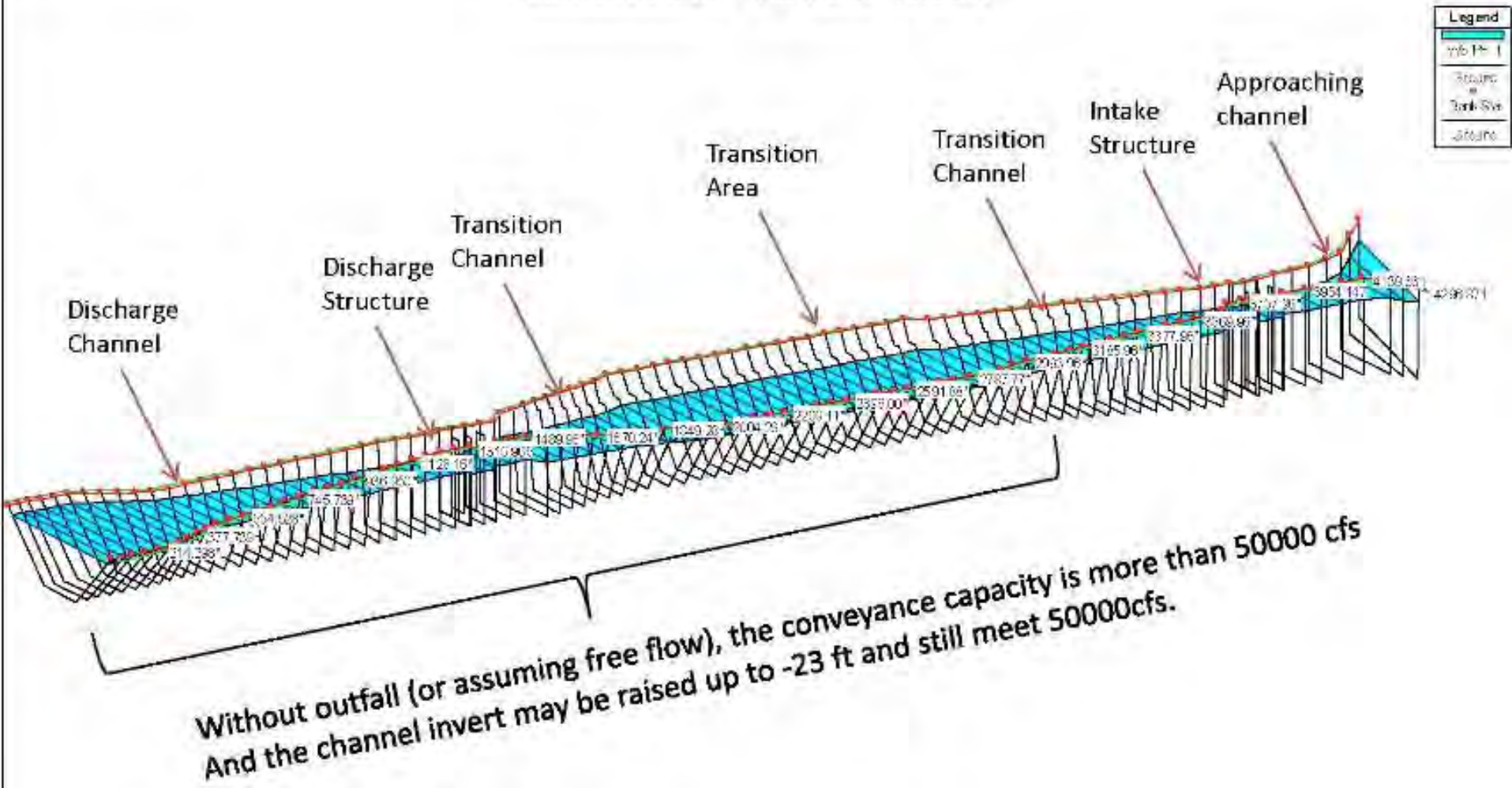
Imagine the result

Questions/Comments



Diamond Short Channel – *without outfall*

DiamondShortDiversion Plan: Plan 02 10/19/2014



Revised Cost Considerations

Cost Estimating

- 50% Concrete, 28% Earthwork, 12% Steel (Piles)
- Re-evaluating 5 unit prices (90% of construction costs)

First Priority (70%)

- Concrete (50%)
 - ✓ Intake Structure (12%)
 - ✓ T-Walls (12%)
 - ✓ Channel Lining (10%)
 - ✓ Outfall Structure (8%)
 - ✓ Stabilization Slabs (8%)
- Excavation (20%)

Second Priority (20%)

- Pipe Piles (7%)
- Sheet Pile (5%)
- Fill (5%)
- Dewatering (3%)